

Fig. 3


Fig. 4
plate flow. The results of computations of the dependences $v w_{0}-p$ are presented in Fig. 4 for the values $\xi<\xi_{\mathrm{K}}(\xi=0.1,0.2,0.25,0.3$ are the lines $1-4)$.

One interesting feature of the solutions obtained for visconlastic deformation problems should be noted in the case of a linear function $\Phi$. The linear dependence of the characteristic rates of deflection on the load (2.5), (2.7) is sufficiently regular for all plates in the presence of one flow mode, however, an analogous dependence in the presence of several zones with moving boundaries is somewhat unexpected. Nevertheless, despite the awkwardness of the analysis, the deviations from the linear dependence are not large in all cases for the known solutions (see [1, 2, 4-6], Figs. 3 and 4), and are remarked only in the domain of load values near the static limit load. For instance, for a circular plate loaded by uniform pressure [4], the deviations from the linear dependence in the whole range of displacement rates do not exceed $1 \%$ of the static limit load.

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## INVERSE PROBLEM OF MEMBRANE DEFORMATION UNDER CREEP CONDITIONS

I. Yu. Tsvelodub

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1. Inverse problems of membrane deformation under creep conditions in a given time in a convex surface for minimal energy expenditures occur, for instance, in analyzing technological equipment for pressure treatment of materials in the creep regime [1].

Let us consider a membrane occupying a domain $S$ in the $x_{1} O x_{2} p l a n e$ that is bounded by the outline $\gamma$ and is being deformed under the action of external forces $q$ normal to its plane and $\mathrm{pk}_{\mathrm{k}}(\mathrm{k}=1,2)$ applied to $\gamma$ and lying in its plane. The equilibrium equations have the form [2]

$$
\begin{equation*}
\frac{\partial \sigma_{h l}}{\partial x_{l}}=0 \quad(k==1,2), \quad h \sigma_{h l} \frac{\partial^{2} w}{\partial x_{h} \partial x_{l}}=-q, \tag{1.1}
\end{equation*}
$$

where $\sigma_{k} Z(k, Z=1,2)$ are stress tensor components, $h$ is the membrane thickness, and $w$ is its deflection. Summation from 1 to 2 is over the repeated subscripts.

The strain tensor components $\varepsilon_{k} \ell(k, Z=1,2)$ are related to the displacement components $u_{k}(k=1,2)$ in the $x_{1} O x_{2} p l a n e$ and the deflection $w$ by the following dependences [2]:

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$$
\begin{equation*}
\varepsilon_{\hbar l}=\frac{1}{2}\left(\frac{\partial t_{k}}{\partial x_{l}}+\frac{\partial u_{l}}{\partial x_{h}}\right) \cdot+\frac{1}{2} \frac{\partial w}{\partial x_{k}} \frac{\partial w}{\partial x_{l}} \quad(k, l=1,2) . \tag{1.2}
\end{equation*}
$$

We consider that the total strains of the membrane material are comprised of elastic strains subject to Hooke's law and creep strain:

$$
\begin{equation*}
\varepsilon_{h l}=a_{h l m n} v_{m n}+\varepsilon_{h l}^{c} \quad(k, l=1, \because), \tag{1.3}
\end{equation*}
$$

where the creep strain rates $\eta_{k} \mathcal{I}=\stackrel{\circ}{\varepsilon}_{\mathrm{c}}^{\mathrm{c}} \mathrm{Z}$ (The dot denotes differentiation with respect to the time t) are potential stress functions

$$
\begin{equation*}
\eta_{k l}=\frac{\partial \Phi}{\partial \sigma_{l l}} \quad(k, l=1,2), \tag{1.4}
\end{equation*}
$$

where $\Phi=\Phi\left(\sigma_{k} \mathcal{l}\right)$ is the creep potential that is a convex homogeneous function of degree $n+1$ in $\sigma_{k} l(k, Z=1,2)[3]$. The function $\Phi=[1 /(n+1)] W$, where $W=\sigma_{k} \eta_{k} n_{k}$ is the specific power of the energy dissipated during creep which implies the convexity of the functions $W=W\left(\sigma_{k Z}\right)$ and $W=W\left(\eta_{k Z}\right)$ [3], for any two states the following inequality holds [4]

$$
\begin{equation*}
W^{(2)}-W^{(1)} \geqslant \frac{n+1}{n} \sigma_{k l}^{(1)}\left(\eta_{h l}^{(2)}-\eta_{h l}^{(1)}\right) . \tag{1.5}
\end{equation*}
$$

Let us formulate the inverse problem whose investigation is the purpose of this paper: What external forces $q=q\left(x_{1}, x_{2}, t\right)$ and $p_{k}=p_{k}(s, t)(k=1,2)$, where $s$ is the arclength of the contour $\gamma, 0 \leqslant t<t_{夫}$, must be applied to a membrane which is in the natural unstrained state at $t<0$ such that given values of residual deflections $w_{\%}=w_{*}\left(x_{1}, x_{2}\right)$ would be obtained at $t=t \%$ after their instantaneous removal and corresponding elastic unloading, and such that the work of these forces expended in deforming the membrane would be minimal? In other words, among all possible loading paths resulting in a given residual surface shape of an initially flat membrane in a given time $t_{*}$, the optimal path in the sense of energy expenditure must be selected.

We consider the given surface to be convex, i.e.,

$$
\begin{equation*}
\frac{\partial^{2} w_{*}}{\partial x_{1}^{2}}<0, \quad \frac{\partial^{2} w_{*}}{\partial x_{1}^{2}} \frac{\partial^{2} w_{*}}{\partial x_{2}^{2}}-\left(\frac{\partial^{2} w_{*}}{\partial x_{1} \partial x_{2}}\right)^{2}>0 \tag{1.6}
\end{equation*}
$$

and also that $w_{*}=u_{k}^{*}=0(k=1,2)$ on $\gamma$, where $u_{k}^{*}$ are residual displacements in the plane of of the membrane.

It can be shown that the creep strain components are compatible for $t=t_{*}$, i.e., relationships of the type of (1.2) are expressible in terms of $u_{k}^{*}(k=1,2)$ and $w_{k}$. In fact, after unloading the field of residual stresses $\sigma_{k}^{*} \mathcal{Z}$ and residual deflection $w_{*}$ should satisfy a system of equations of the form (1.1) for $t=t_{\%}$, in which we should set $q=0$ [5]. If the residual stress function $F_{*}=F_{*}\left(x_{1}, x_{2}\right)$ is introduced in the usual manner such that the first two equations in (1.1) are satisfied identically, then the third equation in (1.1) will take the form

$$
\begin{equation*}
\frac{\partial^{2} w_{*}}{\partial x_{2}^{2}} \frac{\partial^{2} F_{*}}{\partial x_{1}^{2}}-2 \frac{\partial^{2} w_{*}}{\partial x_{1} \partial x_{2}} \frac{\partial^{2} F_{*}}{\partial x_{1} \partial x_{2}}+\frac{\partial^{2} w_{*}}{\partial x_{1}^{2}} \frac{\partial^{2} F_{*}}{\partial x_{2}^{2}}=0 . \tag{1.7}
\end{equation*}
$$

Since $p_{\hat{k}}^{\dot{*}}=0(k=1,2)$ on $\gamma$ for $t=t_{*}$, then the boundary conditionsfor $F_{*}$ can be reduced to the form [6] $\partial F_{*} / \partial x_{k}=0(k=1,2)$ or $F \%=\partial F_{*} / \partial n=0$ on $\gamma$. By virtue of (1.6), the equation (1.7) for the function $F_{*}=F_{*}\left(x_{1}, x_{2}\right)$ is elliptic [7] and on the basis of the boundary conditions mentioned has the unique solution $F_{ \pm}=0$, from which o $\sigma_{k}^{*}=0(k, Z=1,2)$.

There results from (1.2) and (1.3)

$$
\begin{equation*}
\varepsilon_{h l}^{c}\left(\boldsymbol{t}_{*}\right)=\frac{1}{2}\left(\frac{\partial u_{k}^{*}}{\partial x_{l}}+\frac{\partial u_{l}^{*}}{\partial x_{k}}\right)+\frac{1}{2} \frac{\partial w_{*}}{\partial x_{k}} \frac{\partial w_{*}}{\partial x_{l}} \quad(k, l=1,2) \tag{1.8}
\end{equation*}
$$

Let us now calculate the work $A$ expended by the forces $q$ and $p_{k}(k=1,2)$ in membrane deformation under the assumption that $w=0$ on $\gamma$ during the whole process, i.e., for $0 \leqslant t \leqslant$ $t_{\%}$. We have

$$
A=I_{1}+I_{2}, \quad I_{1}=\int_{S} \int_{0}^{w_{*}} q d w d x_{1} d x_{2}, \quad I_{2}=\int_{\gamma} \int_{0}^{u_{k}^{*}} p_{k} d u_{k} d s
$$

By virtue of (1.1) and the known Green's formula that reduces integration over the domain $S$ to integration over the contour $\gamma$, we can obtain

$$
\begin{gathered}
I_{1}=-h \int_{S}^{w_{*}^{*}} \frac{\partial}{\partial x_{l}}\left(\sigma_{k l} \frac{\partial w}{\partial x_{k}} d w\right) d x_{1} d x_{2}+I_{3}=-h \int_{\nu}^{w_{*}} \int_{0}^{w_{k}} \sigma_{k l} \frac{\partial w}{\partial x_{k}} n_{l} d w d s+I_{3} \\
I_{3}=h \int_{S}^{w_{0}^{*}} \int_{0} \sigma_{k l} \frac{\partial w}{\partial x_{k}} d\left(\frac{\partial w}{\partial x_{l}}\right) d x_{1} d x_{2}
\end{gathered}
$$

where $n_{k}(k=1,2)$ are components of the external unit normal vector to $\gamma$. Because of the boundary conditions for $w$ the first integral in the last equality vanishes; therefore, $A=$ $I_{3}+I_{2}$.

It is easy to see that the quantity $A$ equals the work of the stress $\sigma_{k} \ell$ on the strain $\varepsilon_{k l}$ in the whole membrane volume, i.e., $A=h \int_{S}^{\varepsilon_{k l}^{*}} \int_{0}^{*} \sigma_{k l} d \varepsilon_{h l} d x_{1} d x_{2}$, from which, by virtue of (1.3) and the equalities $\sigma_{k}^{*}=0(\mathrm{k}, 乙=1,2)$, we obtain $A=h \int_{S}^{t} \int_{0}^{t *} W d t d x_{1} d x_{2}, \quad W=\sigma_{k l} \eta_{k l}$ 。

Let us prove the following assertion: The optimal loading path (in the above-mentioned sense) is that for which the stress components at each point of the membrane are independent of the time. Such a stress field, if it exists, is uniquely defined.

We assume that such a path exists; all the quantities referring to it will be denoted with the subscript 0 . Then for any other loading, assuring, the given residual deflection $w_{*}=w_{*}\left(x_{1}, x_{2}\right)$ after unloading at $t=t_{*}$, we have

$$
\begin{gather*}
A-A_{0}=h \int_{S} \int_{0}^{t *}\left(W-W_{0}\right) d t d x_{1} d x_{2} \geqslant h \frac{n+1}{n} \int_{S} \int_{0}^{t *} \sigma_{h l_{0}}\left(\eta_{k l}-\eta_{k l_{0}}\right) d t d x_{1} d x_{2} \\
=h \frac{n+1}{n} \int_{S} \sigma_{k l_{0}} \Delta \varepsilon_{h l}^{c}\left(z_{*}\right) d x_{1} d x_{2}=h \frac{n+1}{n} \int_{S} \sigma_{k l_{0}} \frac{1}{2}\left(\frac{\partial \Delta u_{k}^{*}}{\partial x_{l}}+\frac{\partial \Delta u_{l}^{*}}{\partial x_{k}}\right) d x_{1} d x_{2}  \tag{1.9}\\
=h \frac{n+1}{n} \int_{S} \frac{\partial}{\partial x_{l}}\left(\sigma_{k l_{0}} \Delta u_{k}^{*}\right) d x_{1} d x_{2}=\frac{n+1}{n} \int_{\gamma} p_{k 0} \Delta u_{k}^{*} d s=0 . \tag{1.9}
\end{gather*}
$$

In (1.9) we used the inequality (1.5), the condition of independence of $\sigma_{k} \mathcal{Z}_{0}$ from $t$, the relationships (1.8) in which $w_{*}=w_{\star}\left(x_{1}, x_{2}\right)$ is the given function, i.e., $\Delta\left(\frac{\partial w_{*}}{\partial x_{k}} \frac{\partial w_{*}}{\partial x_{l}}\right)=0$ ( $k, Z=1,2$ ), the Green's formula, and the boundary conditions for the residual displacements u** The symbol $\Delta$ denotes the difference between appropriate quantities referring to the loading paths under consideration. Therefore, $A_{0} \leqslant A$, which proves the first part of the
assertion.

The proof of the second part is analogous to the proof of the uniqueness theorem for steady creep problems [8]. Indeed, $\varepsilon_{\mathrm{k}}^{\mathrm{c}} \mathrm{c}^{\left(\mathrm{t}_{*}\right)}=\eta_{\mathrm{k}} \mathrm{t}_{*}(\mathrm{k}, Z=1,2)$ follows from (1.4) and we obtain from ( 1.5 ) by interchanging the roles of the first and second states and combining the inequality obtained with (1.5)

$$
\begin{equation*}
\Delta \sigma_{h l} \Delta \eta_{h l} \geqslant 0, \quad \Delta \sigma_{h l}=\sigma_{h l}^{(2)}-\sigma_{h l}^{(1)}, \quad \Delta \eta_{l l}=\eta_{k l}^{(2)}-\eta_{h l}^{(1)} \tag{1.10}
\end{equation*}
$$

The inequality (1.10) expresses the known Drucker postulate for viscous strain [8]. Assuming the existence of two solutions corresponding to the very same residual deflection w; and satisfying the zero boundary conditions for $u_{k}(k=1,2)$ with time-independent stress fields and performing calculations analogous to those used in (1.9), we find

$$
h \int_{S} \Delta \sigma_{k l} \Delta \varepsilon_{h l}^{c}\left(t_{*}\right) d x_{1} \stackrel{*}{d}_{2}-h t_{*} \int_{S} \Delta \sigma_{k l} \Delta \eta_{k l} d x_{1} d x_{2}=0
$$

which is possible, by virtue of (1.10), if and only if $\Delta \sigma_{k} \mathcal{L}=0(k, \mathcal{Z}=1$, 2) in the whole volume of the membrane since the expression $\Delta \sigma_{k Z} \Delta n_{k} z$ is a positive definite quadratic form in $\Delta_{o_{k}}(k, \tau=1,2)[8]$. The assertion is proved.

The contour loads for a known stress field okl are determined by the dependences $\mathrm{Pk}=$
 determined to find the transverse loads $q=q\left(x_{1}, x_{2}, t\right)$. Eliminating the quantity $u_{k}(k=$ 1,2 ) from (1.2) and taking into account that the creep strain rate $\eta k I(k, Z=1$, 2 ) is independent of $t$, i.e., $\varepsilon_{h l}^{c}(t)=\frac{t}{t_{*}} \varepsilon_{h l}^{c}\left(t_{*}\right)$, by using the relationships (1.3) and (1.8) we obtain the following equality at any time $t\left(0 \leqslant t<t_{*}\right)$

$$
\begin{equation*}
\left.\left(\frac{\partial^{2} w}{\partial x_{1} \partial x_{2}}\right)^{2}-\frac{\partial^{2} w}{\partial x_{1}^{2}} \frac{\partial^{2} w}{\partial x_{2}^{2}}=\frac{1}{E} \Delta \Delta F+\frac{t}{l_{*}}\left[\left(\frac{\partial^{2} w_{*}}{\partial x_{1}}\right)^{2}\right)^{2} \cdots \frac{\partial^{2} w_{*} \partial_{2}}{\partial x_{1}^{2}} \frac{\partial_{1}^{2} w_{*}}{\partial x_{2}^{2}}\right], \tag{1.11}
\end{equation*}
$$

which is the strain compatibility equation [2] for this case. It was assumed for simplicity in the derivation of (1.11) that the membrane material is isotropic, where $E$ is Young's modulus, $F$ is a stress function corresponding to the field $\sigma_{k} \mathcal{L}$, and $\Delta \Delta$ is the biharmonic operator.

The relationship (1.11) is a Monge-Ampere equation in the unknown deflection w. Its Dirichlet problem with the above-mentioned boundary condition $w=0$ on $\gamma$ has a unique solution, at least for a negative right side (The other solution differs just by a sign) [7].

If the time $t_{*}$ is sufficiently large, then the components of the stress ok will evidently be small quantities; consequently, the elastic strains (constant in time) can be neglected in comparison with the developed creep strains, i.e., the steady creep scheme can be used [3]. Then the first term in the right side of (1.11) can be omitted, and the ellipticity condition for this equation can be satisfied by taking account of (1.6), implying the unirueness (to the accuracy of a sign) of its solution [7]. In this case, evidently $w=$ $\sqrt{t / t_{*}} W_{*}$.
2. Let us consider a rectangular membrane with the sides $2 \alpha$ and $2 b, a / b=\varepsilon<1$. Let us select the origin at the center of the membrane, and let us denote the axes by $x$ and $y$ so that the domain $S$ is determined by the inequalities $|x| \leqslant a,|y| \leqslant b$. Let us determine the optimal stress field $\sigma_{k I}$ (constant in time) of which we spoke above, for this case. As the potential $\Phi$ we take the standard [3] from (1.4): $\Phi=\frac{B}{n+1} \sigma_{i}^{n+1}$, where $\sigma_{i}=\sqrt{\sigma_{x}^{2}+\sigma_{y}^{2}-\sigma_{x} \sigma_{y}+3 \sigma_{x y}^{2}}$ is the intensity of the stress $B, n$ are constants, $n>1$.

Let us introduce the dimensionless coordinates $\tilde{x}=x / \alpha, \tilde{y}=y / b$, the displacements $\tilde{u}=$ $u / a, \tilde{v}=v / a$, and the deflection $\tilde{w}=w / \alpha$ in the $x O y$ plane, later discarding the tilde symbol ~ above the dimensionless quantities so that the domain $S$ will be defined by the inequalities $|x| \leqslant 1,|y| \leqslant 1$.

To solve the problem, we apply the method of perturbations [9] by selecting the quantity as small parameter. We shall assume that the given residual deflection $w_{*}=w_{*}(x, y) d e-$ pends only on the dimensionless coordinates and does not contain the parameter $\varepsilon$, where $w_{\psi}( \pm 1$, $y)=w_{*}(x, \pm 1)=0$. For simplicity we consider that $w_{*}$ is an even function in both the variables, i.e., $w_{*}(x, y)=w_{*}(-x, y)=w_{*}(x,-y)$. The residual displacements $u *$ and $v_{*}$ satisfy the zero boundary conditions, i.e., $u_{*}=v_{*}=0$ for $x= \pm 1$ and $y= \pm 1$. We later omit the asterisk subscript $\%$ on the quantities $w_{*}, u_{*}$, and $v_{*}$.

Under the assumptions made for the creep strain components for $t=t_{*}$, we obtain from (1.4) and (1.8)

$$
\begin{gather*}
\varepsilon_{x}^{c}\left(t_{*}\right)=\frac{\partial u}{\partial x}+\frac{1}{2}\left(\frac{\partial w}{\partial x}\right)^{2}=B t_{*} \sigma_{i}^{n-1}\left(\sigma_{x}-\frac{1}{2} \sigma_{y}\right), \\
\varepsilon_{y}^{c}\left(t_{*}\right)=-\varepsilon \frac{\partial v}{\partial y}+\frac{\varepsilon^{2}}{2}\left(\frac{\partial w}{\partial y}\right)^{2}=B t_{*} \sigma_{i}^{n-1}\left(\sigma_{y}-\frac{1}{2} \sigma_{x}\right),  \tag{2.1}\\
\varepsilon_{x y}^{c}\left(t_{*}\right)=\frac{1}{3}\left(\frac{\partial u}{\partial x}+\varepsilon \frac{\partial u}{\partial y}\right)+\frac{\varepsilon}{2} \frac{\partial w}{\partial r} \frac{\partial u}{\partial!}=B t_{*} \sigma_{i}^{n-1} \frac{3}{3} \sigma_{x y}
\end{gather*}
$$

The first two equilibrium equations in (1.1) take the form

$$
\begin{equation*}
\partial \sigma_{x} / \partial x+\varepsilon \dot{\partial} \sigma_{x y} / \partial y=0, \quad \partial \sigma_{x y} / \partial x+\varepsilon \partial \sigma_{y} / \partial y=0 \tag{2.2}
\end{equation*}
$$

Using the usual method [9], we represent the magnitudes of the displacements and stresses in the form of power series in $\varepsilon$ and then isolating terms with identical powers in (2.1) and (2.2). Thus, we have for the zeroth approximation

$$
\frac{\partial u_{0}}{\partial . x}=B t_{*} \sigma_{i 0}^{n-1}\left(\sigma_{x 0}-\frac{1}{2} \sigma_{m 0}\right)-\frac{1}{2}\left(\frac{\partial u}{\partial x}\right)^{2}
$$

$$
\begin{gather*}
0=B t_{*} \sigma_{i 0}^{n-1}\left(\sigma_{y 0}-\frac{1}{2} \sigma_{x v}\right)  \tag{2.3}\\
\frac{\partial v_{0}}{\partial x}=3 B t_{*} \sigma_{i 0}^{n-1} \sigma_{x y 0}, \frac{\partial \sigma_{x 0}}{\partial x}=\frac{\partial \sigma_{x y 0}}{\partial x}=0 .
\end{gather*}
$$

We use the boundary conditions for the variable $x$, i.e., $\left.u_{0}\right|_{x \rightarrow 1}=\left.v_{0}\right|_{x=11}=0$, to solve the system (2.3). Consequently, it is not difficult to obtain

$$
\begin{gather*}
\sigma_{x!}=\sigma_{0}, \quad \sigma_{y 0}-\frac{1}{2} \sigma_{0}, \quad \sigma_{x y y}=0, \quad \sigma_{i 0}=\frac{\sqrt{3}}{2} \sigma_{0}, \\
\sigma_{0}=\left[\frac{1}{2 B t_{*}}\left(\frac{2}{\sqrt{3}}\right)^{n+1} \cdot \int_{0}^{1}\left(\frac{\partial w}{\partial x}\right)^{2} d x\right]^{\frac{1}{n}},  \tag{2.4}\\
u_{0}=\frac{x}{2} \int_{0}^{1}\left(\frac{\partial w}{\partial x}\right)^{2} d x-\frac{1}{2} \int_{0}^{x}\left(\frac{\partial w}{\partial x}\right)^{2} d x, \quad v_{0}=0 .
\end{gather*}
$$

Evenness of the function $w=w(x, y)$ is used in (2.4). Because $w(x, \pm 1)=0$, we obtain $\left.\partial w i \partial x\right|_{y= \pm 1}=0$, from which $u_{0} l_{y=+1}=0$. Therefore, the solution (2.4) of the system (2.3) for the zeroth approximation satisfies all the boundary conditions.

We have the system of first approximation conditions from (2.1) and (2.2):

$$
\begin{gather*}
\frac{\partial u_{1}}{\partial x}=B t_{*}\left(\frac{\sqrt{3}}{2} \sigma_{0}\right)^{n-1} \cdot\left(\frac{3 n+1}{4} \sigma_{x 1}-\frac{1}{2} \sigma_{y 1}\right), \\
\frac{\partial v_{0}}{\partial y}=B t_{*}\left(\frac{\sqrt{3}}{2} \sigma_{0}\right)^{n-1}\left(\sigma_{y 1}-\frac{1}{2} \sigma_{x 1}\right),  \tag{2.5}\\
\frac{\partial v_{1}}{\partial x}+\frac{\partial u_{0}}{\partial y}=3 B t_{*}\left(\frac{\sqrt{3}}{2} \sigma_{0}\right)^{n-1} \sigma_{x y 1}-\frac{\partial w}{\partial x} \frac{\partial w}{\partial y}, \\
\frac{\partial \sigma_{x 1}}{\partial x}+\frac{\partial \sigma_{x y 0}}{\partial y}=0, \quad \frac{\partial \sigma_{x y 1}}{\partial x}+\frac{\partial \sigma_{y 0}}{\partial y}=0 .
\end{gather*}
$$

The solution of the system (2.5) with the boundary conditions $u_{1 \mid x=1}=\left.v_{1}\right|_{x=+1}=0$ : is

$$
\begin{gather*}
\sigma_{x 1}=\sigma_{y 1}=0, \quad \sigma_{x y 1}=-\frac{x}{2} \frac{\partial \sigma_{0}}{\partial y}, \quad u_{1}=0 \\
v_{1}=\left(\frac{1}{n}+\frac{1}{2}\right)\left(1-x^{2}\right) \int_{0}^{1} \frac{\partial w}{\partial x} \frac{\partial^{2} w}{\partial x \partial y} d x+\int_{x}^{1}\left(\int_{0}^{x} \frac{\partial w}{\partial y} \frac{\partial^{2} w}{\partial x^{2}} d x\right) d x . \tag{2.6}
\end{gather*}
$$

The equalities (2.4) were used in (2.6). It is seen that $v_{1} \|_{y= \pm 1}=0$, i.e., all the boundary conditions are satisfied even for the first approximation.

We obtain the system of second approximation equations from (2.1) and (2.2):

$$
\begin{gather*}
\frac{\partial u_{2}}{\partial x}=B t_{*}\left(\frac{\sqrt{3}}{2}\right)^{n-1} \sigma_{0}^{n-2}\left\{\sigma_{0}\left(\frac{3 n+1}{4} \sigma_{x 2}-\frac{1}{2} \sigma_{y 2}\right)+\frac{n-1}{2}\left[\frac{3(n+1)}{4} \sigma_{x 1}^{2}+\sigma_{y 1}^{2}-2 \sigma_{x 1} \sigma_{y 1}+3 \sigma_{x y 1}^{2}\right]\right\}, \\
\frac{\partial v_{1}}{\partial y}+\frac{1}{2}\left(\frac{\partial w}{\partial y}\right)^{2}=B t_{: 3}\left(\frac{\sqrt{3}}{2}\right)^{n-1} \sigma_{0}^{n-2}\left\{\sigma_{0}\left(\sigma_{y 2}-\frac{1}{2} \sigma_{x 2}\right)+(n-1) \sigma_{x 1}\left(\sigma_{y 1}-\frac{1}{2} \sigma_{x 1}\right)\right\}, \\
\frac{\partial v_{2}}{\partial x}+\frac{\partial u_{1}}{\partial y}=3 B t_{*}\left(\frac{\sqrt{3}}{2}\right)^{n-1} \sigma_{0}^{n-2}\left\{\sigma_{0} \sigma_{x y 2}+(n-1) \sigma_{x 1} \sigma_{x y 1}\right\},  \tag{2.7}\\
\partial \sigma_{x 2}\left\{\partial x+\partial \sigma_{x y 1} l \partial y \doteq 0, \quad \partial \sigma_{x y 2} / \theta x+\partial \sigma_{y 1}(\partial y=0 .\right.
\end{gather*}
$$

The solution of the system (2.7) satisfying the boundary conditions $\left.u_{2}\right|_{x= \pm 1}=\left.v_{2}\right|_{x=\frac{1}{2} 1}=0$ is

$$
\begin{gathered}
\sigma_{x 2}=\frac{x^{2}}{4} \frac{\partial^{2} \sigma_{0}}{\partial y^{2}}+\frac{1}{n \sigma_{0}}\left[\frac{1}{2 B t_{*}}\left(\frac{2}{\sqrt{3}}\right)^{n+1} \frac{1}{\sigma_{0}^{n-2}} \int_{0}^{1} \Phi_{1} d x-\Phi_{2}\right], \\
\sigma_{y 2}=\frac{1}{2} \sigma_{x 2}+\frac{1}{B t_{*}}\left(\frac{2}{\sqrt{3}}\right)^{n-1} \frac{\Phi_{1}}{\sigma_{0}^{n-1}}, \quad \sigma_{x y 2}=0, \\
u_{2}=\frac{x}{2} \int_{0}^{1} \Phi_{1} d x-\frac{1}{2} \int_{0}^{x} \Phi_{1} d x+B t_{*}\left(\frac{\sqrt{3}}{2}\right)^{n+1} \sigma_{0}^{n-2} \Phi_{2}\left(x^{3}-x\right), \quad v_{2}=0,
\end{gathered}
$$

$$
\begin{equation*}
\Phi_{1}=\frac{\partial v_{1}}{\partial y}+\frac{1}{2}\left(\frac{\partial w}{\partial y}\right)^{2}, \quad \Phi_{2}=\frac{n \sigma_{0}}{12} \frac{\partial^{2} \sigma_{0}}{\partial y^{2}}+\frac{n-1}{6}\left(\frac{\partial \sigma_{0}}{\partial y}\right)^{2} . \tag{2.8}
\end{equation*}
$$

The following remark must be made with respect to the solution obtained above for the first approximation. As is seen from (2.4), $\left.\sigma_{0}\right|_{y= \pm 1}=0$ which can result in infinite stresses $\sigma_{\mathrm{xy} 1}, \sigma_{\mathrm{x} 2}$, and $\sigma_{\mathrm{y} 2}$ for $\mathrm{y}= \pm 1$. In turn this imposed definite constraints on the applicability of (2.6) and (2.8).

For instance, let $w=\alpha\left(1-y^{2}\right) P_{Q}(x)$, where $\alpha, p$ are constants and $Q( \pm 1)=0$. For the quantities $\partial k_{\sigma_{0}} / \partial y^{k}(k=1,2, \ldots)$ that will be in the expressions for the higher order approximations of the stresses to be finite, it is necessary that $2 \mathrm{p} / \mathrm{n}$ be a natural number. Thus, $p \geqslant n$ follows from the condition of boundedness of the stresses $\sigma_{x y 1}, \sigma_{x 2}$, and $\sigma_{y 2}$ for $y= \pm 1$. In this case $u_{2}(x, \pm 1)=0$, i.e., all the boundary conditions are satisfied for the second approximation also.

Let us consider an example: $n=3, w=\alpha\left(1-y^{2}\right)^{3}\left(1-x^{2}\right)$. We find from (2.4), (2.6), and (2.8)

$$
\begin{gathered}
\sigma_{x}=\beta\left\{\left(1-y^{2}\right)^{2}+\varepsilon^{2}\left[x^{2}\left(3 y^{2}-1\right)+\frac{137 y^{2}+1}{45}\right]\right\}, \beta=\frac{4}{3}\left(\frac{\alpha^{2}}{2 B t_{4}}\right)^{\frac{1}{3}}, \\
\sigma_{y}=\beta\left\{\frac{\left(1-y^{2}\right)^{2}}{2}+\varepsilon^{2}\left[\frac{9}{8} x^{4}\left(7 y^{2}+1\right)-\frac{1}{4} x^{2}\left(189 y^{2}-1\right)+\frac{15263 y^{2}-671}{360}\right]\right\}, \\
\sigma_{x y}=2 \beta e x y\left(1-y^{2}\right), \\
u=\frac{2}{3} \alpha^{2}\left(1-y^{2}\right)^{4}\left(x-x^{3}\right)\left\{\left(1-y^{2}\right)^{2}+\frac{\varepsilon^{2}}{60}\left[9 x^{2}\left(7 y^{2}+1\right)-1087 y^{2}+79\right]\right\}, \\
v=-\alpha^{2} \varepsilon\left(1-x^{2}\right)\left(x^{2}+\frac{5}{3}\right) y\left(1-y^{2}\right)^{5} .
\end{gathered}
$$

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